BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF ANY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.

2. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

3. EQUIPMENT FRAMING, LOADS, OPENINGS AND STRUCTURE IN ANY WAY RELATED TO HYAC, PLUMBING, OR ELECTRICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY, CONTRACTOR SHALL OBTAIN APPROVAL OF THE INVOLVED TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN THESE REQUIREMENTS TO BE BORNE BY THE APPROPRIATE CONTRACTOR.

4. SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

5. GOVERNING CODE: OHIO BASIC BUILDING CODE

6. DESIGN CRITERIA. a. FLOOR LIVE LOADS: 1. OFFICES

> 4. STAIRWAYS 100 PSF 5. MECHANICAL ROOM 150 PSF

A DEAD LOAD ALLOWANCE OF 20 PSF FOR PARTITIONS HAS BEEN ADDED FOR FLOOR AREAS WITH A DESIGN LIVE LOAD LESS THAN 80 PSF.

b. ROOF LOADING I. DESIGN ROOF LIVE LOAD 25 PSF 2 ROOF SNOW LOADS! a. GROUND SNOW LOAD b. FLAT-ROOF SNOW LOAD C. SNOW EXPOSURE FACTOR d. SNOW IMPORTANCE FACTOR 1.0

. WIND LOADING: BASIC WIND SPEED 2. WIND IMPORTANCE FACTOR 1.0 . WIND EXPOSURE CATEGORY a MAIN WIND FORCE RESISTING SYSTEM B b. COMPONENTS AND CLADDING

4. WIND DESIGN PRESSURES. a. MAIN WIND FORCE RESISTING SYSTEM 20 PSF b. COMPONENTS AND CLADDING

d EARTHQUAKE DESIGN DATA: (NOT APPLICABLE, WIND LOADING GOVERNS)

B. FOUNDATIONS

I. THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE SURVEY AND THE SUB-SURFACE INVESTIGATION REPORT BEFORE BEGINNING CONSTRUCTION.

2. NOTIFY THE ARCHITECT AS SOON AS POSSIBLE OF ANY UNUSUAL SOIL CONDITIONS OR SOIL CONDITIONS IN VARIANCE WITH TEST BORINGS, SUCH AS UNEXPECTED SPRING OR SEEPAGE WATER, MATERIAL DIFFERING FROM TEST BORINGS, OR SOIL OF QUESTIONABLE BEARING CAPACITY

3. ALL FOOTINGS SHALL BEAR ON FIRM UNDISTURBED SOIL OR ENGINEERED FILL WITH AN ALLOWABLE BEARING CAPACITY OF 2,500 POUNDS PER SQUARE FOOT.

4. PERIMETER FOOTINGS SHALL BEAR A MINIMUM OF 3'-O" BELOW EXTERIOR GRADE; STEP FOOTINGS AS REQUIRED. WHERE UNDERGROUND UTILITIES ENTER OR EXIT THE BUILDING, STEP TOPS OF ALL FOOTINGS BELOW UTILITY INVERT ELEVATIONS.

5. BACKFILL AND FILL MATERIALS: PROVIDE SATISFACTORY SOIL MATERIALS FOR BACKFILL AND FILL, FREE OF CLAY, DEBRIS, WASTE, FROZEN MATERIALS, VEGETABLE AND OTHER DELETERIOUS MATTER.

6. EXCAVATION AND COMPACTION.

a IT SHALL BE THE OWNER'S RESPONSIBILITY TO RETAIN THE SERVICES OF A SOILS ENGINEER TO INSPECT AND APPROVE FOUNDATION EXCAVATIONS FOR THE BEARING CAPACITY INDICATED ABOVE. THE CONTRACTOR SHALL COORDINATE (THROUGH THE OWNER) THE SCHEDULING OF THE SOILS ENGINEER'S SERVICES WITH THE ANTICIPATED DATE OF CONCRETE PLACEMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL FOUNDATION MODIFICATIONS AS RECOMMENDED BY THE OWNER'S SOILS ENGINEER b. KEEP FOUNDATION EXCAVATIONS FREE OF WATER AT ALL TIMES. REPLACE SOFT

OR WEAKENED SOIL WITH LEAN CONCRETE (CLASS IV). . THE EXISTENCE OF UNDERGROUND STRUCTURES AND/OR UTILITIES IS NOT KNOWN IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE (THROUGH THE OWNER) THE LOCATIONS OF ALL EXISTING UNDERGROUND STRUCTURES AND/OR UTILITIES. USE EXTREME CARE WHEN EXCAVATING SO AS NOT TO DISTURB ANY EXISTING UNDERGROUND STRUCTURES AND/OR UTILITIES.

d. BACKFILL AND FILL SHALL BE PLACED IN LIFTS OF 8" MAXIMUM LOOSE DEPTH. EACH LIFT SHALL BE COMPACTED WITH A POWER VIBRATING COMPACTOR OR SIMILAR EQUIPMENT TO ASSURE MAXIMUM COMPACTION OF THE MATERIAL. e. COMPACTION SHALL BE NOT LESS THAN 100% OF MAXIMUM DENSITY FOR COHESIVE OR COHESIONLESS MATERIAL, ACCORDING TO ASTM D698. DRAINAGE FILL AGAINST

WALLS SHALL BE COMPACTED TO 96% OF MAXIMUM DENSITY 7. WHERE LOOSE FILL MATERIALS ARE ENCOUNTERED, THE LOOSE MATERIALS SHALL BE OVER-EXCAVATED DOWN TO SUITABLE SOILS. THE OVER-EXCAVATED AREA SHALL THEN BE PROOF-ROLLED AND FILLED WITH SATISFACTORY SOIL MATERIALS WHICH WILL PRODUCE THE RESULTS OF COMPACTION AND LOAD CARRYING CAPACITY REQUIRED. THE FILL SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH THE ABOVE PARAGRAPHS.

C. REINFORCED CONCRETE

I. ALL CONCRETE MORK SHALL BE IN ACCORDANCE WITH ACI 318-89, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."

2. SPECIFICATIONS: IN GENERAL, COMPLY WITH ACI 301-89, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

3. ALL REINFORCING DETAILS SHALL CONFORM TO ACI 315-86, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.

4. MATERIALS: a STRUCTURAL CONCRETE:

CLASS LOCATION FOOTINGS AND PIERS INTERIOR SLABS ON GRADE AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED

EXTERIOR SLABS ON GRADE AND ALL EXTERIOR CONCRETE NOT (with air) OTHERWISE IDENTIFIED

BACKFILL BELOW FOOTINGS FILL OVER METAL CENTERING AND 3500 FILL IN STAIR PANS AND TREADS (#8 aggregate)

b, ALL DEFORMED REINFORCING BARS: ASTM A615, GRADE 60. C. ALL WELDED WIRE FABRICI ASTM A185, DELIVERED IN FLAT SHEETS.

5. FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.

a. DONELS IN FOOTINGS TO MATCH VERTICAL PIER REINFORCING.

b. PROVIDE CORNER BARS AT ALL FOOTING CORNERS TO MATCH HORIZONTAL REINFORCING. MINIMUM LENGTH OF EACH LEG - 45 BAR DIAMETERS. C. BACKFILL AGAINST BOTH SIDES OF WALLS EQUALLY UNTIL THE LOWER ELEVATION

d FROT BEGEAN CONCRETE (CLASS IV) UNDER FOUNDATIONS FOR ACCIDENTAL OVER-EXCAVATION, SOFT SPOTS AND TRENCHES.

7. CONSTRUCTION JOINTS: a. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR AS APPROVED BY THE STRUCTURAL ENGINEER. ALL CONSTRUCTION JOINTS ARE TO BE KEYED.

8. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE MINIMUM CONCRETE COVER AS FOLLOWS: a. CONCRETE CAST AGAINST AND PERMANENTLY

EXPOSED TO EARTH b. CONCRETE EXPOSED TO EARTH OR WEATHER! #5 BARS AND SMALLER

C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER,

1. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH THE FOLLOWING REFERENCES AND STANDARDS: a. AMERICAN CONCRETE INSTITUTE, COMMITTEE 530. b. NATIONAL CONCRETE MASONRY ASSOCIATION.

2. MATERIALS a. CONCRETE BLOCK, ASTM C90. b. MORTAR: TYPE S, MINIMUM COMPRESSIVE STRENGTH: 1800 PSI. C. BOND BEAM AND CORE FILL: ASTM C476, COARSE TYPE, MINIMUM COMPRESSIVE STRENGTH: 3000 PSI.

d. JOINT REINFORCING: MILL GALVANIZED FINISH, 9 GAGE MINIMUM SIDE WIRES AND CROSS WIRES e. BAR REINFORGING: ASTM A615, GRADE 60.

3. REINFORCED MASONRY: WHERE VERTICAL BARS ARE TO BE GROUTED INTO CORES, THE FOLLOWING REQUIREMENTS APPLY a. PROVIDE DOWELS FROM FOOTING, SAME SIZE AND SPACING AS WALL BARS. LAP 12 INCHES MINIMUM WITH WALL BARS. EMBED INTO FOOTING 8 INCHES PLUS STANDARD 90 DEGREE HOOK.

b. PROVIDE A CONTINUOUS VERTICAL CAVITY, AT LEAST 2" X 3" IN SIZEFREE OF PROVIDE AN OPENING FOR CLEANOUT AND INSPECTION AT EACH VERTICAL BAR, AT BOTTOM OF EACH GROUT LIFT. d. PROVIDE REBAR ALIGNMENT DEVICES AT A MAXIMUM SPACING OF 192 BAR

e. AT SPLICES IN VERTICAL BARS, PROVIDE MECHANICAL COUPLERS OR 48 BAR DIAMETER LAP. f. MAXIMUM HEIGHT OF GROUT LIFT = 4'-0".

4. MISCELLANEOUS.

DIAMETERS

a. VERTICAL COLLAR JOINTS TO BE FILLED SOLID WITH MORTAR. b. PROVIDE 100% SOLID BEARING, MINIMUM THREE COURSES UNDER BEAMS, AND COLUMNS c. FILL CORE SOLID AROUND ANCHOR BOLTS. d. HOLLOW MASONRY UNITS TO BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL

AND VERTICAL FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES OF PIERS, IN THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAID WITH FULL HEAD AND BED JOINTS. e. PROVIDE JOINT REINFORCING AT 16 INCHES, EXCEPT AS NOTED f. LAP JOINT REINFORCING 6 INCHES.

E. STRUCTURAL STEEL

a. STRUCTURAL STEEL: ASTM A36, FY = 36 KSI; HIGH STRENGTH BOLTS: ASTM A325 OR A490; ANCHOR BOLTS: ASTM A307 OR A36; ELECTRODES: SERIES E70; STRUCTURAL PIPES. ASTM A53 OR A501, FY=35 KSI MIN; SQUARE AND RECTANGULAR TUBING: ASTM A500, FY = 46 KSI; EXPANSION BOLTS: HILTI "KWIK-BOLTS" OR APPROVED EQUAL.

2. SPECIFICATION: WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AMS DI.I. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION TO BE GOVERNED BY: a. AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF

STRUCTURAL STEEL FOR BUILDINGS (JUNE 1, 1989). b. AISC CODE OF STANDARD PRACTICE (SEPTEMBER 1, 1986) C. STRUCTURAL WELDING CODE, ANS DIJ-88 OF THE AMERICAN WELDING SOCIETY. d. SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS (NOVEMBER 13, 1985).

a. FIELD CONNECTIONS TO BE BOLTED. SHOP CONNECTIONS TO BE WELDED OR

5. MISCELL ANEOUS

3. CONNECTIONS:

BOLTED. CONNECTIONS TO BE DESIGNED BY THE FABRICATOR TO DEVELOP THE FULL UNIFORM LOAD STRENGTH OF MEMBER, FOLLOW INSTRUCTIONS ON DRAWINGS FOR GENERAL ARRANGEMENT OR PARTICULAR DETAILS.

a. DO NOT PAINT STEEL OR ANCHOR BOLTS WHICH WILL BE ENCASED IN CONCRETE OR ANY STEEL WHICH WILL BE LOCATED INSIDE THE FINISHED PRODUCT CONCEALED FROM VIEW. LINTELS AND COLUMNS IN EXTERIOR WALLS ARE TO RECEIVE TWO COATS OF SHOP PAINT

a. PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS, OBTAIN PRIOR APPROVAL

b. STEEL SUPPORTING OR CONNECTING TO ROOF OPENINGS, HVAC AND OTHER EQUIPMENT AS SHOWN ON DRAWINGS IS SHOWN FOR BIDDING PURPOSES ONLY CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION BEFORE PROCEEDING WITH HIS WORK . PROVIDE NON-SHRINK, NON-METALLIC GROUT UNDER ALL BEARING PLATES AND BASE

PLATES. STEEL BELOW GRADE TO BE PROTECTED BY A MINIMUM OF 3 INCHES OF CONCRETE OR 4 INCHES OF MASONRY

e. PROVIDE 1/4 INCH THICK SETTING PLATES FOR ALL BEAMS BEARING ON MASONRY J. PREFABRICATED WOOD TRUSSES. OR CONCRETE WHICH DO NOT REQUIRE A BEARING PLATE. 1. PROVIDE SHOP WELDED ANCHORS FOR ATTACHMENTS OF MASONRY. SPACING TO BE 24 INCHES ON BEAMS, 16 INCHES ON COLUMNS. 9 PROVIDE HEAVY NUT AND WASHER AT ALL ANCHOR BOLTS (BOTH ENDS WHERE

h. FINISH ENDS OF ALL COLUMNS, STIFFENERS AND ALL OTHER MEMBERS IN DIRECT

I. MINIMUM BEAM BEARING ON MASONRY = 8 INCHES UNLESS NOTED OTHERWISE. J. EMBEDMENT LENGTH OF EXPANSION BOLTS INTO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS: 1/2 INCH DIAMETER BOLTS --- 3-1/2 INCHES EMBEDMENT 3/4 INCH DIAMETER BOLTS --- 5 INCHES EMBEDMENT

F. METAL DECKING

. REFERENCE STANDARDS a. SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 1980 EDITION, BY THE AMERICAN IRON AND STEEL INSTITUTE b. DESIGN MANUAL FOR FLOOR DECKS AND ROOF DECKS, BY THE STEEL DECK

2. MATERIALS: STEEL SHEET CONFORMING TO ASTM A446-85 OR A611-85.

3. GALVANIZED FINISHES: CONFORM TO ASTM A525-86, G60

4. ACCESSORIES, SAME MATERIAL AND FINISH AS DECK UNITS.

5. ATTACHMENT TO SUPPORTING MEMBERS: ATTACH TO SUPPORTS BY WELDING FROM TOP SIDE ONLY. WELDS WHICH BURN HOLES IN DECKING OR SUPPORTING MEMBER WILL BE REJECTED, MINIMUM SIZE AND SPACING OF WELDS TO BE AS RECOMMENDED BY THE

G. STRUCTURAL LINTELS

. THE FABRICATOR SHALL PROVIDE LINTELS OVER ALL OPENINGS IN MASONRY WALLS REFER TO ARCHITECTURAL AND HVAC DRAWINGS FOR LOCATION, NUMBER AND SIZES OF OPENINGS. FOR LINTELS LABELED ON STRUCTURAL DRAWINGS, USE THE APPROPRIATE LINTEL FROM THE LINTEL SCHEDULE BELOW AND APPLY NOTES 4 THROUGH 6. FOR LINTELS NOT LABELED OR SHOWN ON STRUCTURAL DRAWINGS APPLY NOTES

2. FOR LINTELS OVER OPENINGS 6'-O" WIDE OR LESS, PROVIDE THE FOLLOWING FOR EACH 4 INCHES OF WALL THICKNESS (USE 6" MINIMUM BEARING EACH END); IN CAVITY WALLS, ADD CONTINUOUS BOTTOM PLATE 5/16" X (WALL "T"-1/2") AND STOP PLATE 1/8" SHORT OF JAMBS.

MASONRY OPENING SECTION REMARKS TO 4'-0" L3-1/2 X 3-1/2 X 5/16 4'-1" TO 5'-6" L4 X 3-1/2 X 5/16 LLV 5'-7" TO 6'-0" L5 X 3-1/2 X 5/16 LLV 6'-1" TO 10'-0" L6 x 3-1/2 x 5/16 LLV FOR BRICK VENEER ONLY 10'-1" TO 14'-0" L7 X 4 X 3/8 LLV FOR BRICK VENEER ONLY

3. FOR LINTELS OVER OPENINGS GREATER THAN 6'-O" WIDE, PROVIDE THE FOLLOWING BEAM SECTIONS WITH 8" MINIMUM BEARING EACH END: ADD CONTINUOUS BOTTOM PLATE 5/16" X (WALL "T"-1/2") AND STOP PLATE 1/8" SHORT OF JAMBS:

MASONRY OPENING SECTION 6'-1" TO 6'-6" WBXB 6'-T" TO 7'-11" W 8 X 18 8'-0" TO 12'-0" W8 X 21

4. REFER TO SECTION 16/S4 FOR WIDE FLANGE BEAM LINTELS.

5. REFER TO SECTION 15/54 FOR MASONRY BEARING DETAIL FOR LINTELS.

H. LIGHTGAGE METAL FRAMING

I. MATERIALS AND FINISHES: a HEADERS AND JOISTS: ASTM 446, GRADE C OR D. MINIMUM YIELD STRESS

B. ALL OTHER MATERIALS: ASTM 446, GRADE A. MINIMUM YIELD STRESS 33 KSI C. GALVANIZED FINISH PER ASTM A525, 660.

2. SPECIFICATIONS: ALL LIGHTGAGE METAL FRAMING SHALL BE IN ACCORDANCE WITH AISI SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS,

3. STUDS: ALL STUDS USED FOR EXTERIOR LOAD BEARING WALLS AT THE FIRST FLOOR AND INTERIOR LOAD BEARING WALLS AT THE SECOND FLOOR SHALL BE 6" DEEP STEEL "C" STUDS WITH THE FOLLOWING MINIMUM PROPERTIES

GAGE AREA (in.2) 1x (in.4) 5x (in.3) 1y (in.4) 5y (in.3) 20 0.346 1.808 0.538 0.118

10 12

a. STUDS SHALL BE INSTALLED SO ENDS HAVE FULL BEARING AGAINST INSIDE TRACK WEBS. SECURELY ATTACH ENDS OF STUDS TO BOTH FLANGES OF UPPER AND LOWER TRACKS. DO NOT SPLICE STUDS.

MANUFACTURER'S RECOMMENDATIONS FOR SPACING, EXCEPT THAT THE MAXIMUM SPACING IS TO BE 3'-4" FOR BEARING WALLS AND 5'-0" FOR ALL OTHER WALLS. PROVIDE HEADERS AND SUPPORTING STUDS AS REQUIRED AT ALL OPENINGS IN WALLS. JAMB SECTIONS SHALL CONSIST OF HEAVIER GAGE STUDS, MULTIPLE STUDS, OR BOTH, AS REQUIRED TO CARRY THE HEADER END REACTION AND/OR THE WIND LOADS FOR THE ADJACENT OPENING.

a. PROVIDE WEB STIFFENERS AT ENDS OF EACH MEMBER. b. PROVIDE CONTINUOUS TRACKS AT TOP AND BOTTOM OF ALL HEADERS.

b. INSTALL LATERAL BRACING TO PREVENT STUD ROTATION. USE THE

a. FASTENING OF COMPONENTS SHALL BE DESIGNED BY THE FABRICATOR, FASTENING SHALL BE WITH SELF-DRILLING SCREWS OR WELDING. SCREWS AND WELDS SHALL BE SUFFICIENT IN NUMBER AND SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. ALL WELDS SHALL BE TOUCHED-UP WITH A ZINC RICH PAINT. b. SPLICES IN FRAMING MEMBERS OTHER THAN END TRACKS SHALL NOT BE PERMITTED.

6. SIZE AND GAGE OF END TRACKS SHALL MATCH SIZE AND GAGE OF FRAMING MEMBERS, UNLESS NOTED OTHERWISE.

I. STRUCTURAL LUMBER

a. STRUCTURAL LUMBER: Fb = 1,200 PSI, Fv = 90 PSI, Fc = 1,000 PSI, Ft = 625 PSI, E = 1,600 KSI, Fc (PERP. TO GRAIN) =

405 PSI, MAX. M.C. = 19%. I. ROOF: C-C PLUGGED, 1/2 INCH, APA STRUCTURAL I RATED SHEATHING, 24/0, EXPOSURE I (WITH PLYWOOD CLIPS). 2. WALL: 1/2 INCH, OSB, APA STRUCTURAL I RATED SHEATHING, 16" O.C.,

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF. a. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS

b. U.S. PRODUCT STANDARD PS-I FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL

a. RAFTERS TO BEAMS: 16 GA. GALVANIZED STD. JOIST HANGERS, UNLESS SHOWN b. PLYWOOD TO ROOF TRUSSES OR RAFTERS: NAILED, USE 6d RING SHANK NAILS AT 6 INCHES O.C. AT PANEL EDGES AND 12 INCHES O.C. AT INTERMEDIATE

SUPPORTS, PROVIDE PLYWOOD CLIPS AT MID-SPAN OF PLYWOOD BETWEEN SUPPORTS C. OSB TO STUDS: SCREWED, USE TYPE S-12 SCREWS AT 12" O.C. EACH STUD AND AT 6" O.C. ALONG TOP AND BOTTOM TRACK.

d. PROVIDE SIMPSON POST BASE & POST CAP AT TOP & BOTTOM OF ALL WOOD POSTS.

a. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8"-O" O.C. MAX. FOR ALL RAFTERS. USE SOLID BLOCKING AT RAFTER BEARING. b. PROVIDE A DOUBLE PLATE AT THE TOP OF ALL METAL STUD FRAMED WALLS. APPLY CONTINUOUS BEAD OF GLUE ON GROOVE OF TONGUE-AND-GROOVE PANELS.

5. LVL INDICATES MICROLAM MEMBER BY THE TRUS-JOIST CORP.

a. LUMBER: SPECIES PER DESIGN BY TRUSS MANUFACTURER; NO. 2 GRADE OR BETTER

b. CONNECTIONS: ALL INTERNAL CONNECTIONS ARE TO BE DESIGNED BY THE TRUSS SUPPLIER. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A446, GRADE A COATING GLASS GGO PER ASTM A525. MANUFACTURE WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED. . HANGERS: ALL TRUSS-TO-TRUSS HANGERS ARE TO BE PROVIDED BY THE TRUSS

a. ROOF LOADS:

TOP CHORD LIVE LOAD: 25 PSF TOP CHORD DEAD LOAD: 10 PSF BOTTOM CHORD LIVE LOAD: 20 PSF BOTTOM CHORD DEAD LOAD: 5 PSF

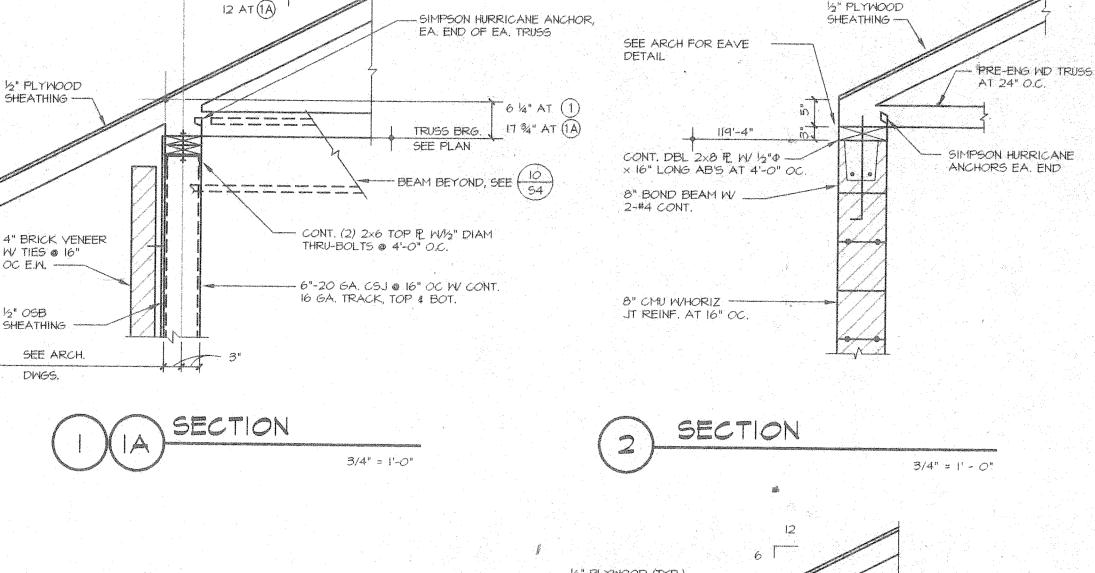
NET WIND UPLIFT: 8 PSF b. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN OHIO, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE MANUFACTURER . DESIGN BOTTOM CHORD OF GIRDER TRUSSES FOR THE END REACTION OF SUPPORTED TRUSSES. THE DESIGN OF ALL HANGER CONNECTORS SHALL BE THE

3/4" = 1'-0"

RESPONSIBILITY OF THE TRUSS SUPPLIER

a. SUBMIT TRUSS SHOP DRAWINGS WHICH EXHIBIT THE SEAL OF THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN b. SUBMIT LAYOUT DRAWING WHICH INDICATES THE LOCATION OF EACH TRUSS

. SUBMIT HANGER CONNECTOR TYPES AND LOCATIONS.



PRE-ENG WD

SIMPSON HURRICANE ANCHOR

EA. END, EA. TRUSS

6x20 GA CSG

SEE 1/95 FOR

ADD'L INFO

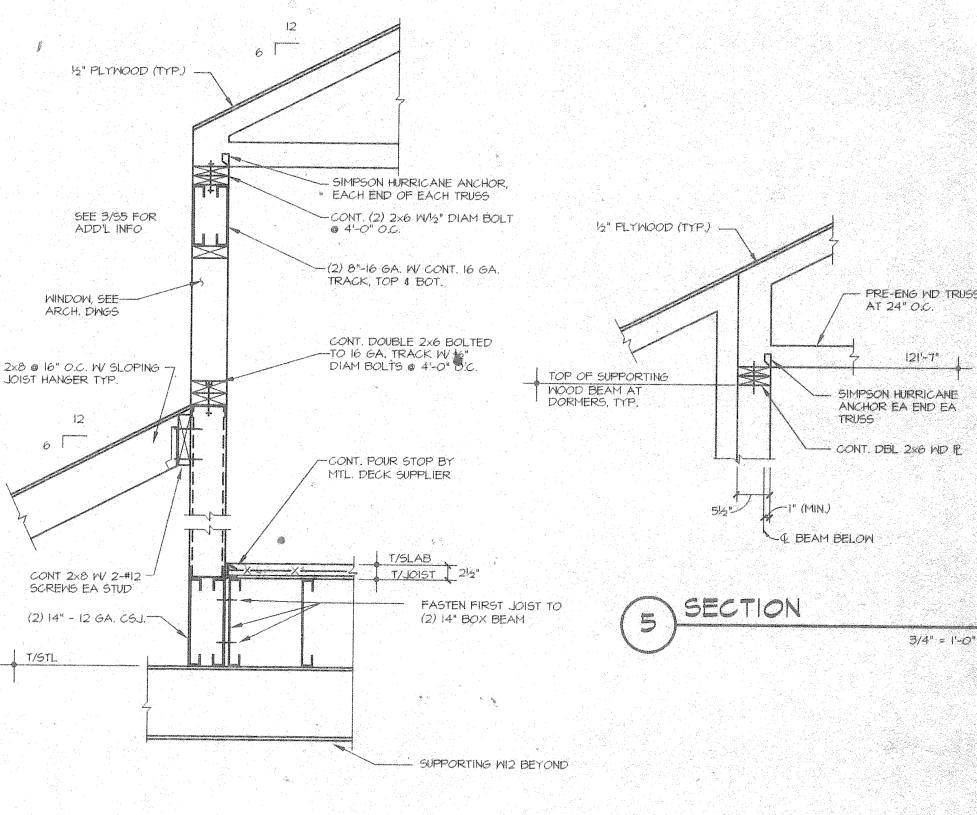
AT 16" OC W/ CONT

16 GA TOP TRACK

3/4" = 1'-0"

TRUSSES AT 24" OC

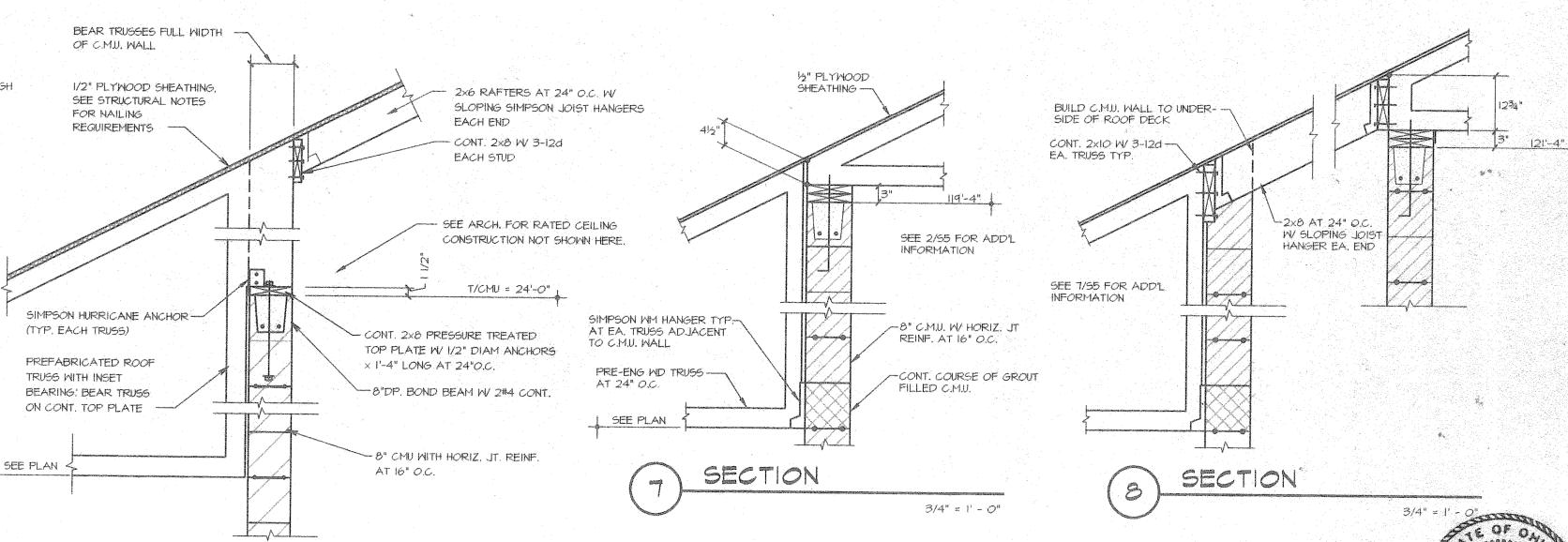
3'' = 1'0''



3/4" = 1'-0"

WILLIAM SHIRK

52389



1'' = 1'0''

5" PLYWOOD (TYP.) -

1/2" OSB SHEATHING -

CONT. 2x6 W/ 2-#12

SCREWS EA. STUD.

SLOPE W ROOF

SHEATHING

15" PLYWOOD -

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PROJECTMANAGER

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KEAIEMED BA

written consent of the architect.

AS NOTED

DATE 7-15-96

NOTES

JASONWAY

MED OFFICE

STRUCTURAL

NOTES & SECTIONS 55